



Special Report 79-29

August 1979

ADA 080649

MASS WATER BALANCE DURING SPRAY IRRIGATION WITH WASTEWATER AT DEER CREEK LAKE LAND TREATMENT SITE

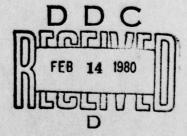
G. Abele, H.L. McKim and B.E. Brockett

DDC FILE COPY

Prepared for
U.S. ARMY ENGINEER DISTRICT
HUNTINGTON, WEST VIRGINIA
By



UNITED STATES ARMY
CORPS OF ENGINEERS
COLD REGIONS RESEARCH AND ENGINEERING LABORATORY
HANOVER, NEW HAMPSHIRE, U.S.A.





Approved for public release; distribution unlimited

80 2 14 032

Unclassified

REPORT DOCUMENTATION	ON PAGE	READ INSTRUCTIONS BEFORE COMPLETING FORM
REPORT NUMBER	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
Special Report 79-29		
TITLE (and Subtitle)		5. TYPE OF REPORT & PERIOD COVERE
MASS WATER BALANCE DURING SPRAY WASTEWATER AT DEER CREEK LAKE L	IRRIGATION WITH	@ Special repto,
TREATMENT SITE	AND	6. PERFORMING ORG. REPORT NUMBER
AUTHOR(e)		8. CONTRACT OR GRANT NUMBER(*)
Gunars Abele, Harlan L. McKim • Bruce E. Brockett		Intra-Army Order No. E 8678ED-03
PERFORMING ORGANIZATION NAME AND ADDR	RESS	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK WHIT NUMBERS
U.S. Army Cold Regions Research	and	19 110
Engineering Laboratory		(42/78)
Hanover, New Hampshire		Section 10 Control Con
1. CONTROLLING OFFICE NAME AND ADDRESS		12 REPORT DATE
U.S. Army Engineer District	(1.	Aug 979
Huntington, West Virginia	,	13. NUMBER OF PAGES
4. MONITORING AGENCY NAME & ADDRESS(II dit	Insent from Controlling Office	15. SECURITY CLASS. (of this report)
The state of the s		SECURITY CEASS. (of this report)
(14) CRRELI-SR.	-79-29	Unclassified
1 - 1 CICK COLL	and and	15a. DECLASSIFICATION/DOWNGRADING
		SCHEDULE
6. DISTRIBUTION STATEMENT (of this Report) Approved for public release; dis	stribution unlimite	
6. DISTRIBUTION STATEMENT (of this Report) Approved for public release; dis-		d •
Approved for public release; dis		d •
Approved for public release; dis		d . m Report)
Approved for public release; dis		d •
Approved for public release; dis		d . m Report)
Approved for public release; dis		d . m Report)
Approved for public release; dis		d . m Report)
Approved for public release; dis		d . m Report)
Approved for public release; dis	ered in Block 20, if different from	d. m Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse side if necessar	ered in Block 20, if different from	d. m Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse elde if necessariants) Sewage treatment	ered in Block 20, if different from	d. m Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse elde if necesses Sewage treatment Waste management	ered in Block 20, if different from	d. m Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse elde if necesses Sewage treatment Waste management	ered in Block 20, if different from	d. an Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse elde if necesses Sewage treatment Waste management	ered in Block 20, if different from	d. m Report)
Approved for public release; dis 7. DISTRIBUTION STATEMENT (of the abstract enter 8. SUPPLEMENTARY NOTES 9. KEY WORDS (Continue on reverse elde if necessariants) Sewage treatment	ered in Block 20, if different from	d. an Report)
Approved for public release; disconnection of the abstract entermandation of the abstract ent	The form and identify by block number) and identify by block number) est area was calculated and the soil from soil samp.	ated during and two days on of wastewater. By computile water content data, cal-
Approved for public release; disconnection of the abstract enterior enteri	The form and identify by block number) est area was calculated as a solution of the form and alculate the water is a solution of the form and alculate the water is a solution of the form and alculate the water is a solution of the form and alculate the water is a solution of the form and a solution of the form and a solution and a solution and a solution of the form and a solution and a solut	ated during and two days on of wastewater. By comput le water content data, calmeasuring the underdrain

Unclassified
SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

PREFACE

This study was conducted by Gunars Abele, Research Civil Engineer, Applied Research Branch, Experimental Engineering Division, and by Dr. Harlan L. McKim, Soils Scientist and Bruce E. Brockett, Physical Science Technician, Earth Sciences Branch, Research Division, USACRREL.

Applying wastewater, monitoring climatological and soil tension instrumentation, obtaining underdrain flow data (after 4 August), and sampling water quality were done by Richard Cantor, Graduate Student, Ohio State University, under the general supervision of Dr. Shamsher S. Brar, Post Doctoral Research Associate, and Dr. Robert H. Miller, Professor, Department of Agronomy, Ohio State University.

This work was performed during 1978 under U.S. Army Engineer District-Huntington, Huntington, WV, Intra-Army Order No. E 8678ED-03.

Appreciation is expressed to the staff of the Deer Creek Lake Corps of Engineers Office at Mt. Sterling, Ohio, for their support and assistance during the 1978 test season. Appreciation is also expressed to Carolyn Merry and James Martel of CRREL and to Professor William Larson of the University of Minnesota for the review of this report and their valuable comments and suggestions.

	ion For
NTIS	Take 1
DDC TA	
Unanno	L-mad
Justif	ication
Ву	
Distri	bution/
Avail	lability Codes
	Avail and/or
Dist.	special
1	The state of the s
a	
	the public business of the

CONTENTS

<u> </u>
Abstract
Preface
Introduction
Description of study
Wastewater application schedule
Observation and test schedules
Density and specific gravity
Soil tension
Water content
Underdrain flow
Climatological conditions
Infiltration test
Other tests
Discussion of results
Sensitivity analysis
Field data
Soil density and specific gravity
Soil water content
Drainage rate
Evapotranspiration
Water budget analysis
Infiltration rate
Summary and conclusions
Literature cited
Appendix: Soil tension vs time and depth

ILLUSTRATIONS

Figur	re	Page
1.	Deer Creek land treatment site	2
2.	Location of field measurements	2
3.	Wastewater application and observation schedule	5
4.	Block diagram of volume relationships	16
5.	Relationship between dry density, gravimetric water content,	
	volumetric water content, and volume of water in soil	22
6.	Volume of water vs. increase in water content	22
7.	Dry density profile of test area	24
8.	Specific gravity profile of test area	24
9.	Gravimetric water content before application	24
10.	Volumetric water content before application	24
11.	Gravimetric water content profiles before, during, and	
	after application	26
12.	Mean gravimetric water content profiles of test area	26
13.	Mean volumetric water content profiles of test area	27
14.	Volumetric composition of test area	27
15.	Drain flow rate compared with wastewater applied and	
15.	rain	29
16.	Drain flow rate vs. time	29
17.	Cumulative volume of water drained vs. time	30
		37
18.	Volume relationships and water budget (2 Aug 1978)	
19.	Infiltration rate	37

TABLES

Table		1
	Application schedule	
2.	Dry density data	
	Soil tension data	
	Water content data	
5.	Underdrain flow data	
6.	Climatological data	
7.	Cumulative drainage	
8.	Results of calculations for water remaining in soil	
9.	Summary of water budget	

NOMENCLATURE

Total area of spray field (three 3-acre plots = 9 acres = A_T 3.64 ha) h = Depth (cm) Total effective depth of spray fields 2 depth of underdrains 2 ≈ 80 cm = Total volume of spray field $(A_T \times h_T \approx 7,700,000 \text{ gal} \approx 29,145,000 \text{ ℓ} \approx 29,000 \text{ m})$ v_T = Dry density of soil (g cm^{-3}) Υ Density of water (assume 1 cm³ = 1 g) = Specific gravity of soil Gs $w = \frac{W}{W} \times 100$ Gravimetric water content (%) Ww Weight of water (g) Dry weight of solids (g) Volumetric water content (%) $V_w = w \frac{\gamma}{\gamma}$ Volume of solids (%) = $\frac{Y}{G_S} \times 100$ Volume of voids (%) Vv Volume of air (%) $V_w + V_a = V_v$ Saturation (%) $S = \frac{V_w}{V_w} \times 100$ S Change in gravimetric water content (%) Δw = Change in volumetric water content (%) AV.

V

V (soil) = Amount of applied water remaining in soil (gal or 1)

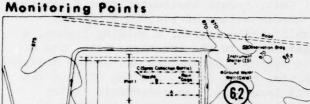
 V_w (soil) = $\Delta V_w \times V_T$

INTRODUCTION

The Deer Creek Lake land treatment system, located approximately 48 km (30 mi) southwest of Columbus, Ohio, treats wastewater from a camping site. The facility, designed to handle a flow of 174,000 £ per day (46,000 gpd) is composed of a stabilization lagoon, a holding lagoon, a pumping system that transports wastewater to the treatment site, and a rotating nozzle spray distribution system that applies wastewater uniformly over four 1.21 ha (3-acre) test plots, using nozzle spacing of 12 m (40 ft) longitudinally and 18 m (60 ft) laterally. An underdrain system with a lateral spacing of 9 m (30 ft) collects the percolate water at a depth of approximately 75 to 80 cm (approximately 30 in) and terminates at a point where the discharge can be diverted either to the stabilization lagoon or to Deer Creek Lake (Fig. 1). The 4 test plots were planted with reed canarygrass, an oat-soybean double crop, alfalfa, and tree seedlings (Fig. 2).

The treatment system was designed by the Huntington District, U.S. Corps of Engineers. The operation and performance of the system have been described by Lambert and McKim (1977).

The primary objective of this study was to determine the total water mass balance during wastewater application. The parameters measured under field conditions will serve as input for available moisture flow models.



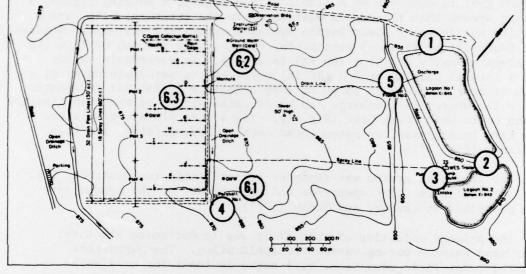


Figure 1. Deer Creek land treatment site.

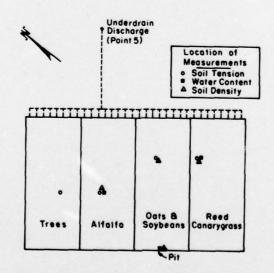


Figure 2. Location of field measurements.

DESCRIPTION OF STUDY

Wastewater Application Schedule

During the summer of 1978, wastewater was applied to the test area a total of 11 times. The amount of application was usually 2.6 cm (slightly over 1 in) of water; for a 3.64-ha (9-acre) area this was equivalent to approximately 946,300 ℓ (250,000 gal).

The first application included all four 1.21-ha (3-acre) plots (grass, oats, alfalfa, and trees). No wastewater was applied to the tree plot during the remaining 10 applications. During the first two applications, the double crop plot contained oats. During the third and fourth applications, the plot contained no crops; the oats were harvested before the third application. The fifth application, immediately after planting of the soybeans, was limited to the grass and alfalfa plots. The last six applications included all three plots with the agricultural crops. Table 1 shows the application schedule, as well as the planting and harvesting dates.

The rate of application, dictated by the spray system's capacity, was approximately 0.5 cm (0.2 in) per hour. The first six applications were done without interruptions, requiring approximately 5 hours. The last five applications included several 15 to 30 min. interruptions to permit soil tension and water content observations during the spraying process. Applications Nos. 9 and 10 were done on a regular 1/2 hour on, 1/2 hour off basis (refer to Fig. 3).

Observation and Test Schedules

Density and specific gravity. The dry soil density and specific gravity G data are listed in Table 2. The 1974 data are from the Ohio State Soil Testing Laboratory records. All other data were obtained by CRREL personnel during the last two years. The specific gravity data from the alfalfa plot are considered to be representative of the entire test area.

Soil tension. Tensiometers were installed at four depths in each of the grass, oat-soybean, and alfalfa plots at the beginning of the test season. Readings were taken daily (with a few exceptions) throughout the season, from 14 June through 15 Sep 1978. During four of the last five applications, soil tension data were also obtained at several intervals during wastewater application (Fig. 3). The data are listed in Table 3 and plotted in the Appendix. Tensiometer locations are shown in Figure 2.

Table 1. Application schedule (1978)

TO BE THE RESERVE OF THE PERSON OF THE PERSO			
No.	Date	Amount applied and crop stage	Area
	18 Apr11	Planting of oats	
	10 June	1st cutting of alfalfa and grass	
1.	14 June	1,283,100 & (339,000 gal)	4.86 ha (12 acres)
2.	22 June	957,600 £ (253,000 gal)	3.64 ha (9 acres)
	27 June	Cutting of oats	
3.	29 June	1,067,000 & (281,900 gal)	п п
4.	5 July	962,100 £ (254,200 gal)	" "
	11 July	Planting of soybeans	
5.	12 July	657,100 & (173,600 gal)	2.43 ha (6 acres)
	17 July	2nd cutting of alfalfa and grass	
6.	25 July	970,900 £ (256,500 gal)	3.64 ha (9 acres)
7.	2 Aug	990,900 £ (261,800 gal)	Man in a second in
8.	10 Aug	615,100 £ (162,500 gal)	
	12 Aug	3rd cutting of alfalfa and grass	
9.	15 Aug	946,300 £ (250,000 gal)	u se sa u
10.	22 Aug	" "	H 18323 385 H
11.	7 Sep	The market have accessed as a contract of the	otto w 1108 w

Beginning with the third application, the wastewater was spiked by adding ammonium nitrate at a rate of 20 mg/liter prior to spraying.

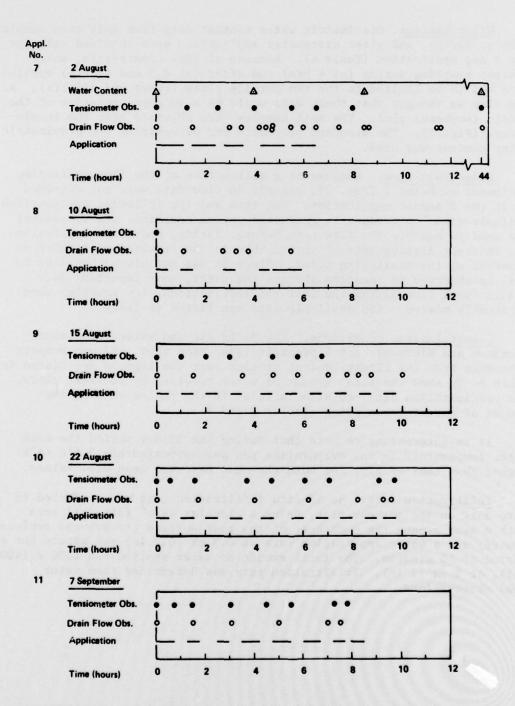


Figure 3. Wastewater application and observation schedule.

Water content. Gravimetric water content data from soil core samples before, during, and after wastewater application were obtained only for the 2 Aug application (Table 4). Because of time constraints, water content sampling during (at 4 hrs) and after (at 8.5 and 44 hrs) application had to be limited to the two outside plots (grass and alfalfa). At the time we thought that these data would be also representative of the middle (soybean) plot. The soil samples were obtained near the tensiometers (Fig. 2). The standard procedure for determining the gravimetric water content was used.

Underdrain flow. Because of a malfunction of the flow monitoring equipment at Point 5 (Fig. 2), underdrain flow data were not obtained until the 2 August application. For this and the following applications, a simple manual procedure (a graduated volume container and stopwatch) was used to measure the flow rate before, during, and after application. The inherent disadvantage of this method is the requirement to have an observer at the monitoring point. Since it was not always possible to have an observer on location through the night, some important data points (peak flow during the last two applications, for example) were apparently missed. The available data are listed in Table 5.

Climatological conditions. The daily air and water temperature (maximum and minimum), total precipitation, mean wind, and pan evaporation data from the climatological station near the lagoons are listed in Table 6. To show the total amount of water received by the test plots, the precipitation data are also included in the tables showing the amount of wastewater applied (Tables 3 and 5).

It is interesting to note that during the 55-day period the mean water temperature in the evaporation pan was approximately 3.5°C (6°F) higher than that of air, for both the mean max. and mean min. values.

Infiltration test. An in-situ infiltration test was conducted in late July on the soybean plot, using a circular 29-m² (314-ft²) area with a seal around the periphery of the test surface (to prevent surface runoff) and a water application rate of 0.5 cm (0.2 in) per minute for a period of 10 minutes. The total amount of water applied was 1500 £ (400 gal), or 5 cm (2 in). Infiltration rate was determined from water head observations.

Table 2. Dry Soil Density and Specific Gravity Data

Gr	ass	Soy	beans	Al	falfa	
Depth (cm)	Density (g cm ⁻³)	Depth (cm)	Density (g cm ⁻³)	Depth (cm)	Density (g cm ⁻³)	G _S
June	1977	May	1974	May	1974	
10 22 39 74	1.71 1.68 1.40 1.51	10 52 122 160	1.56 1.68 1.93 1.86	9 25 55 127	1.69 1.62 1.69 1.81	
April	1978	April	1978	Apri	1 1978	
30	1.41	42 67 111	1.41 1.76 1.71	4 19 39 65 93	1.53 1.51 1.63 1.92 1.95	
		July	1978	Jun	e 1978	
		2.5 7.5 12.5 17.5 22.5 27.5 34 50 62 72 82 88.5 93.5 103.5 108.5 113.5	1.42 1.53 1.75 1.72 1.53 1.66 1.48 1.58 1.64 1.60 1.52 2.02 1.94 1.72 1.74 1.90 1.88	19 39 65 90	1.55 1.67 1.77 1.87	2.67 2.73 2.74 2.76

Table 5. Soil Tension Data (Tensiometer gage readings, cm of water)

Marker applied Laline La											Depth	Depth, in (cm)					
139,015 1.05 (2.64) 1.06	ž			Poto			Reed Can	arygrass			Oats -	Soybeans			T	falfa	
100 100	1		-	in (c	î	5(13)	10(25)	18(46)	30(76)	5(13)	10(25)	25 (64)	.35(89)	5(13)	10(25)	22 (56)	30(76)
			1.05(2.67)	_		810	067	810	,	810	580	610	300	260	220	62	•
252,260 1.06(2.64) 1.66 2.7				_	(.03)	102	370	80	,	140	120	80	20	20	07	20	•
232,960 1.04(2.64) 210 2	16 "			.01	(.03)	160	320	80	,	120	120	80	25	07	42	07	•
252,960 L04(2.64) 30 (7.8) 70 410 100 - 77 360 90 40 230 170 100 (7.8) 100 (7.8) 100 - 7 20 36 90 40 250 170 100 (7.8) 100 100 - 7 20 36 90 40 250 170 100 100 100 100 100 100 100 100 10	17 "			0	(0)	,	•	•		1		•				•	•
1.05 (2.15) 6.0 100 40 -	. 81			0	(0)	20	410	100	,	11	360	06	07	250	170	09	1
252,960 1.05(2.64) 20	. 61			1.05	(2.67)	09	100	07	,	20	20	80		20	20	80	•
2522,960 1.06(2.64) 230 (5.5) 2	20 "			.05	(:13)								,				•
252,960 1.04(2.64) 39 (.78) 120 120 60 -	21 "			.20	(.51)			•			1	•		'		•	•
281,860 1.05(2.67) 1.05 (2.08) 39 30 20 - 440 40 40 0 0 0 0 0 0 0 0 0 0 0 0 0	22 "	252,960	1.04(2.64)		(9/.)	120	120	09	,	210	180	70	10	09	20	20	•
	23 "				(.05)	30	30	20	1	07	07	07	0	0	0	0	•
	24 "			0	(0)	150	100	70	,	170	100	70	0	20	07	80	•
281,860 1.16(2.95) 380 140 60 -	25 "			0	(0)	230	100	20	,	480	160	80	0	140	80	20	•
281,860 1.16(2.95) 610 220 80 -	97		0	.12	(30)	380	140	09	•	750	340	09	20	290	120	07	•
281,860 1.16(2.95) (0 (0)) 780 400 80 20 (Oats cut previous day) 380 260 260 380 400 80 40 50 380 400 50 380 400 50 380 400 50 380 400 50 100 100 100 100 100 100 100 100 10	12			.12	(30)	610	220	80		820	580	90		520	220	80	•
281,860 1.16(2.95) 0 (0) 200 400 80 40 - 840 80 40 50 380 40				.17	(.43)	919	300	80	20		(Oats cut	previous da	(A)	380	260	70	•
		281,860	1.16(2.95)		(0)	780	400	80	70		840	80	04	20	380	09	•
154,220 1.05(2.67) 1.05(2				0	(0)	09	150	09	20	20	087	09	10	20	07	0	•
154,220 1.05(2.67) 1.05(2	I Jul			0	(0)	180	190	09	20	20	240	70	20	100	70	30	•
1554,220 1.05(2.67) 0.10 0.0				.74	(1.88)	,					•	•		,		1	•
154,220 1.05(2.67) 01 (0.0) 60 60 40 0 30 60 40 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				.29	(-74)	30	07	07	0	0	07	09	0	0	0	0	•
254,220 1.05(2.67) .01 (.03) 100 80 50 0 70 100 60 30	. 4				(0)	09	09	07	0	30	09	07	0	20	10	0	•
173,580 1.05(2.67) 1.05 (2		254,220	1.05(2.67)		(:03)	100	80	20	0	20	100	09	0	09	30	20	•
173,580 1.07(2.72) 1.05	: 9			.02	(.05)	30	04	20	•	•	0	0	0	0	0	0	•
173,580 1.07(2.72) 1.07 1.00 60 20 60 120 50 0 120 60 120 60 120 60 120 60 120 60 120 60 120 60 120 60 120 60 120				10.	(.03)	•	20	20	0	09	09	07	0	07	20	0	•
				0	(0)	160	100	09	20	09	120	20	0	120	09	20	•
173,580 1.07(2.72) 0.01 (.03) 580 200 80 20 620 70 0 580 160 160 173,580 1.07(2.72) 0.01 (.03) 580 580 40 (Soybeans planted) 700 340	. 6			0	(0)	320	120	09	20	70	340	09	0	350	100	30	•
173,580 1.07(2.72) 0 (0) 840 80 40 (Soybeans planted) 700 34	. 0			9.	(.03)	280	200	80	20	0	620	70	0	580	160	07	•
173,580 1.07(2.72) 0 (0) 840 620 60 40 (No application) 780 520 170 (No application) 780	=			10.	(.03)	260	360	80	07		(Soybean	s planted)		700	340	09	•
256,480 1.05(2.67) 3.0 (7.3) 2.0 (20) 2.0 (20) 3		173,580	(1.07(2.72)	0	(0)	840	620	09	07		(No app	lication)		780	520	20	•
256,480 1.05(2.67) 1.15 (1.01) 2.00 2.00 2.00 2.00 2.00 2.00 2.00 2.0	13	-		60.	(.23)				,					,	•	•	•
256,480 1.05(2.67) 310 (.03) 120 100 80 20 60 80 80 80 80 80 80 80 80 80 80 80 80 80	. 4			.20	(.51)	,	•	•							•		•
256,480 1.05(2.67) .15 (1.37)	115			10.	(:03)	120	100	80	20					09	80	100	•
" 256,480 1.05(2.67) .15 (.18)	91			0	0		, '							330	160	07	•
" 256,480 1.05(2.67) .15 (.18) 0 820 200 40 0 160 20 0 760	- 1			0			Gras	8 cut							Alfa	1fa cut	
10 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	. 56	1 087 956	105/2 673		100		000			1 0		1 5	. :		1		•
Control of the contro	. 92	200	(10.7)	25	18)		070	700	9		,	160	70	0	160	80	07
	" 12					0	770	0	07			. 6	, 6		1 5		

* Precipitation since previous observation.

Table 3 (cont). Soil Tension Data (Tensiometer gage readings, cm of water)

										ochtu, in (cm)	(6.0)					
Dete	Since		Applied.	Rain		Reed Canarygrass	rygrass			Soybean	aun:			Alfalfa	ılfa	
	Applic.		(Gel) in (cm)	In (Cm)	\$(13)	10(25)	18(46)	30(76)	\$(13)	10(25)	25(64)	35(89)	\$(13)	10(25)	22 (56)	30(76)
28 July 29 : :				0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0												
į					175	385	98	45		(No data obtained)	bt ained)	-	255	200	59	35
	1.25	001.100			92	385	110	65					295	220	9	*
	2.5	96.911			22	385	105	55					310	230	8	8
	4.0	000,001	_		22	375	100	0,4					320	230	8	8
	5.5	207.017	. 96 (2.10)		23	305	8	98					350	27.5	8	42
•	3	201,162	1.0/(2.12)		•	120	01	•					•	260	20	•
•	0.0				•	25	io	0					0	210	•	۰0
	19.0			(60.)10.	21	\$\$	22	•					•	01	•	ô
	21.0				20	\$	92	•					•	2	•	ô
•	23.5				35	20	07	9					2	•	21	ê
::	64.0			.05(.13)	95		2 3	25	3	91	S	•	20	25	25	
				(0)0	8		9		2	9	3	00	00			
				(60.)10.	8		• •	2	28	9	9	00	98	2 0	200	
• 2	•	162,500	.69(1.65)	.46(1.17)	2 8	2 2	2 3	0 0	2 80	88	33	00	90	0 0	0 0	00
	: 3			.30(.76)	200		۰ ۾	• •	• •	00	00	00	00	00	00	
2:				.01(90		2 5	25	• •	0	0 9	00	00	0	•	
18		1	3	i i			20	30	260	188	04	0	200	3	3	1
	-	25,000	or i		180	8	9	98	230	100	9	•	200	9	9	•
	~	20,000			\$	100	70	20	200	100	9	•	220	3	3	2
	•	75,000			9	98	9	8	120	901	9	•	240	00	9	10
		100,000	.41(1.04)		•	•	•	•	1		•		•	•	•	•
	4.75				20	9	3	8	0,	00	3	•	20	20	20	۰
	5.75	150,000	-		•	\$	9	20	8	30	9	•	•	•	•	•
•	6.75	175,000	(20.83)		•	20	20	•	20	20	20	•	•	•	•	•
• •		225.000	.92(2.34)		•	•	•	•	•	•	•	•	'	•	•	•
	7.6	250,000	1.02(2.59)	3	' '	-		-			.	. !	1			
				(61.)	-	•	-	>	•	•	•	0	•	•	•	_

Table 3 (cont.) Soil Tension Data (Tensiometer gage readings, cm of water)

	7 e e e									Depth,	Depth, in (cm)					
:	Start of	Commission,	active	In (ca)		Reed Ca	Reed Canarygrass			Soyb	Soybeans				Alfalfa	
1	(hrs)	i			1913	10(25)	18(46)	30(26)	Sun.	(52)0	15(64)	15(80)	\$(13)	10(25)	22(56)	30(76)
10 Aug.					_	08	9	25	120	09	07	0	2	3	50	•
				(6.)(1.	320	88	2 '	33	260	180	28	•	360	130	2 8	2 2
					_	. :	• :	13				1		•	1	i
		25.000	.1 (.25)		_	047	3	2		200	3	•	200	180	8	3
	9.0				009	260	8	3	·	360	9	•	280	180	9	20
	1.6	30,000	(10.) 7.		009	260	90	09	•	380	09	0	580	280	9	20
	:	100,000	.4 (1.02)		971	280	•	5		380	9	•	660	220	4	00
		125,000	.5 (1.27)		!			3	l.		3	•	1	1	3	•
	3	150 000	4 (1.53)		9	240	2	3		320	3	•	2	220	2	•
	3				20	220	90	99	•	380	9	•	•	160	•	٠
	7.1	175,000	(87.1)		•	220	80	04		340	09	0	•	100	•	
		225,000	.9 (2.29)								3		,	1		
	:	250 000	1000		•	160	2	•	•	120	0,	•	•	•	•	•
	6.7	300,000			_	140	3	0			•	•	0	•	20	
3	72			(60.)10.	-	9	3	2	20	07	2	•	0	0	•	Ī
	3			000	_	8	3:	23	2	9	9;	0	• ;	0	•	•
	1			.38(.97)	_	07	3 9	200	8 2	9 9	3 9	0 0	2 -	9 =	9 0	
				.30(.76)	_		•	•	•		•			• •	•	
				1.52(3.86)	•		•	•		•	•	•	•	•	•	and the
				(80.)(0	_	2	9	20.	9	9	, 02				. 6	
				.01(.03)	-	3 .	3 '		3 '	3 '	: '	٠,		: '	3 '	
				.06(.15)	_	100	3	20	140	901	9	0	180	9	3	_
				.01(.03)	_	8	9 8	25	260	8	9 :	•	220	9 :	3:	• ;
		-			_	200	2 9	9	280	220	3 8		2095	2 9	3 9	2
		37,500	.15(.36)		_		1		2	1	3	,		3	}	
	:	20.00			909	220	2	9	260	220	98	•	30	8	8	0
	1.5	20,000	(10.)		009	220	98	04	570	220	90	•	320	90	9	20
		87,500	.35(.89)		307	000	•	*		*	:			;		
	;	125,000	.5 (1.27)		200	077	8	2	000	077	2	•	97	-	2	•
	4.5				280	240	90	8	510	230	02	10	20	90	9	•
•	5.5	mc*701	(co.1)co.		220	240	90	0,	480	220	20	•	20	9	9	
		200,000	.8 (2.03)		5	91.	8				:		;	;	;	
	3	225,000	.9 (2.29)		2	2.	2	2		200	2	•	20	9	3	
	7.8				•	220	9	0*		02	9	•	•	0,	9	Ī
	8.5	250,000	1.0 (2.54)					•				-				Total Control
	77				_	140	09	_ 07	20	50	0,0	0	20	97	20	-
• •	3:			.01(.03)	8 8	140	38	200	120	02	8	0	8	2	2	•
	"				_	330	2 5	2 5	300	120	00 :	0	210	9	3	*
: .					770	300	3 2	2 2	250	9 2	9 9	00	230	9	25	- ;
				,	670	380	68	9	009	180	2	20	250	2	2 9	2
				,	260	450	70	07	220	076	80		079	10	200	
									2	2	25	,	242	2	20	

Table 4. Water content data.

19.0 15.9 17.7	Alf h Depth (cm)	Water Cont. (%)	h Depth (cm)	w Water Cont
19.0 15.9 17.7	Depth (cm)	Water Cont.	Depth	Water Cont
19.0 15.9 17.7	(cm) 4	(\$)		
15.9 17.7		10.7		
15.9 17.7	5	10.3	2.5	14.8
		19.6	7.5 12.5	14.2
16 2	10	15.9	12.5	11.9
16.2	14	15.9	17.5	12.2
15.9	15.5	16.7	22.5	22.1
				18.0
15.6				26.2
				25.1
				22.6
				24.6
				26.6
				11.4
			93.5	12.3
	81.5	11.8		
11.5				
After	4 hrs			
22.0	7	19.7		
	68	12.5		
13.0	80	14.7		
After	8.5 hrs			
19.1	7	16.6		
	22			
21.1	36			
15.6	45	17.1		
12.2	50	12.4		
After	44 hrs			
20.0	•	10.4		
	20.4 21.0 21.0 23.4 17.0 21.1 19.9 14.1 15.3 13.4 11.3 11.5 After 22.0 17.5 21.3 18.0 15.5 13.0 After	15.6 20.4 20.4 20.4 21.0 32 21.0 35 23.4 42 17.0 51.5 21.1 51.5 19.9 60.5 14.1 73.5 15.3 11.5 After 4 hrs 22.0 7 17.5 22 21.3 37 18.0 53 15.5 68 13.0 After 8.5 hrs 19.1 7 18.4 22 21.1 36 15.6 45 12.2 50 After 44 hrs	15.6 24 21.6 20.4 22.4 21.0 32 25.3 21.0 35 23.9 23.4 42 21.1 7.0 51.5 17.1 21.1 51.5 16.6 19.9 60.5 13.9 14.1 73.5 10.8 15.3 81.5 11.8 11.5 11.5 11.5 11.5 11.5 11.5	15.6 20.4 20.6 20.4 20.6 21.0 32 25.3 62 21.0 35 23.9 72 23.4 42 21.1 82 17.0 51.5 17.1 88.5 21.1 51.5 16.6 93.5 21.1 17.0 60.5 13.9 14.1 17.5 15.3 11.4 11.3 11.5 After 4 hrs 22.0 7 19.7 17.5 22 21.2 21.3 37 25.4 18.0 53 14.6 15.5 68 12.5 13.0 80 14.7 After 8.5 hrs 19.1 7 16.6 18.4 22 13.7 21.1 36 15.5 15.6 45 17.1 12.2 50 12.4 After 44 hrs

Table 5. Underdrain flow data (Point 5)

į	Her of Application	303	theer Applied. Commistive (gal) in (cs)	La (ca)	Plow Rate (ge)/min)	Page 1	Time Since Start of Applic. (ire)	(a) o the	Mater Applied, Communitive (gel) in (cm)	(B)	(al/la)
į	.55	66,000	.27(.69)	(0)0	212		26.5			((1,)80.	122
	23:	151,500	.62(1.57)		22		92				12
	333	183,300	.75(1.91)		11.2	: : : 5 2 2				(4.); (4.);	- 6 6
	23	237,100	.97 (2.46)		23.8	22	9.	80.00	.20(.51)	-	5.5
	3:	261,800	1.07 (2.72)		\$8.0		6.3	150,000	.41(1.04)		22
	22				33		9.5	200,000	.92(2.06)		9.9
	7 5				25.7		9.5	237,500	1.02(2.59)		3.
				(60.)10.	12.5	2 % %	22			.01(.03)	2.5
	1833			(61.)80.	233						:
				1.5 (0.8)		11.				(%) %(.	3232
8 8				.01(.03)	2 Z 2 9					.04(.15) .00(.06) .00(.00)	2.7
<u> </u>	0.2			((1.1))	3,2					\$6. (6.) 10.	223
	33:	25,98 20,08 20,08	36. 26.		22.7		e :	80,000	.30(.51)	000	0.0
	122	137,500	.31(.79)		, 52 ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° ° °	•••	2.00	137,500	. 56(1.42)		121
-	23	1000,200	(80.11)00.	(97.)06.	36.2		7.5	250,000	1.02(2.59)		7
	69.69			(60.)10.			3\$2			6.00	200
-	0	1	10 700	6000	19:	:::					9.5
	22:	100,000	710.00		1:	23					• •
	2.5	155,000	.61(1.55)		11.8	. 2				•	6.3
	7.0	200,000	.82(2.08)		11.5						
	8.5	200,000	.82(2.08)		\$0.4						

Table 6. Climatological data

Note: Data shown below were obtained each day at approx. 9 am. Therefore, the data obtained on a particular date actually represent the climatological conditions for the 24-hour period prior to observation. (Net evaporation = evaporation + precipitation; negative pan evap. indicates evap.

Date of observ.	Air temp. max.	(OF) min.	Water to	mp. (°F) min.	Wind Mean (mph)	Precipitation (in)	Pan eva- poration (in)	
June							3.707	
14	83	42	95	50		-		
15	82	43	87	55		.01	.31	.32
16	72	43	76	58		.01	.47	.48
17	86	43	85	58		0	.22	.22
18 19	81 89	61	88 92	62 66	4.2	1.05	(60)	.34
20	81	55	93	65	0.3	.05	.22	.27
21	82	62	93	67	1.0	.20	.05	.25
22	80	77	88	57	1.3	.30	.18	.48
23	74	48	89	56	0.6	.03	. 25	. 28
24	80	55	93	60	0.5	0	.23	.23
25 26	87 84	52	94 91	60 67	0.8	.12	.18	.18
27	88	65	90	68	4.7	.12	.31	.43
28	94	63	97	70	3.9	.17	.31	.48
29	87	69	99	73	1.0	.01	.23	. 24
30	90	61	97	69	1.3	0	.44	.44
Mean:	84	56	91	62	1.9	.13	.21	. 34
July	87	61	94	68	1.3	0	.24	. 24
2	88	63	86	67	1.3	.74	(46)	. 28
2 3 4	74	63	76	65	0.4	.29	(26)	.03
4	71	60	75	64	0.6	0	.07	.07
5	76	54	86	63	0.6	.01	.17	.18
6	76	55	87	63	0.4	.02	.03	.05
7	85 87	60 65	94 94	64 67	1.2	0.01	.35	.36
9	87	55	91	65	3.5 1.8	0	.21	.21
10	87	64	93	66	2.4	0	.25	.24
11	81	48	87	57	2.8	0	.19	.19
12	76	46	88	56	1.3	0	.33	.33
13	79	55	91	57	1.2	.09	.10	.19
14 15	81 89	57 61	86 94	63 67	2.4	.20	(06)	.14
16	84	56	90	66	1.2	0.01	.37	. 38
17	80	49	89	58	2.5	0	.29	.20
18	83	51	93	62	1.0	0	.31	.31
19	85	55	93	64	1.0	.01	.23	. 24
20	90 93	63	96	66	1.1	0	.26	.26
21	99	68 67	99 93	72 73	1.2	0	.28	.28
22 23 24	93	65	94	71	3.8	0	.24	.24
24	92	64	92	69	3.8	.54	(08)	.46
25	77	60	80	69	0.3	.15	(05)	.10
26	77	64	80	70	1.6	.07	.01	.08
27 28	88 83	56 56	93 97	69 64	4.0	0	.29	.29
29	82	58	92	64	0.8	1 0	.14	.14
30	83	60	93	64	3.4	.27	(27)	0
31 Mean	76 83.5	59	92	65	0.1	.37	(28)	.09
				La Ta	1.0			
Aug	79	55	80	60	0.8	.02		
2	84	56	87	61	2.2	0.02	.15	.15
2 3 4 5 6	86 88	64 54	90 85	69	2.2 2.1 2.3 2.0	.01	.25	. 26
4	88	54	85	60	2.3	.05	.24	.29
5	78	55	85	59	2.0	0 58	.17	.17
7	98	54	85 85 83	59 60	0.4 0.1 3.6	1.5		
8	81	56		61		.01	.18	.19
Mean		56	85	61	1.7	.27		.18
Total (55			-50			7.02		12.93
Mean	83.2	57.7	89.6	63.5		.13		. 25

A 1.2 m high, 46-cm-diameter core was obtained from the area adjacent to the infiltration test site for further laboratory tests and analyses. The pit which was excavated to obtain the large core was used to make a detailed description of the soil profile, particularly soil structure and root distribution with depth.

Other tests. Water quality tests, including chemical and biological analyses, analyses of plant samples for nitrogen, phosphorus, potassium and heavy metal content and crop yield determinations were conducted by Ohio State University and will be discussed in a separate report.

The Surgeon General's Office conducted a "virus in, virus out" study in the lagoons during early August.

DISCUSSION OF RESULTS

Sensitivity Analysis

During the data reduction and water mass budget analysis, it became obvious that very small variations in the field measurements or in the computed values, depending on the method of data analysis, can result in very significant variations in the water budget values.

The computation of the changes in the soil water content resulting from wastewater application, and the computation of the amount of percolation into the underdrain system requires integration of areas under irregular curves or a sequence of straight lines connecting discrete data points. Since the applied water volume is relatively small in comparison with the total soil volume of the spray fields and the existing water in the soil, a small change in the soil moisture content represents a significant portion of the applied water volume. Therefore, the computation of the water budget values is very sensitive to the accuracy of the field measurements and to the manner in which the data representing the soil characteristics (density, specific gravity, volume of water and solids) are plotted and interpolated during computations.

Examples of the influence of measurement, analysis, or computational variations (or errors) in the 1) water content, 2) dry density of soil, 3) volumetric water content, and 4) specific gravity of solids on the water budget values are shown below.

Total volume of field, $V_T \sim \frac{7,700,000 \text{ gal}}{\text{underdrains was approximately 80 cm}}$ (three 3-acre plots = 3.64 ha, 80 cm deep; the depth of the underdrains was approximately 80 cm)

Typical volume of water applied, $V_{\text{w(appl)}} \sim \frac{250,000 \text{ gal}}{250,000 \text{ gal}}$ (which is equivalent to 2.6 cm of water on a 3.64 ha area, or 3.25% of V_{T})

Typical mean values of the spray field soil profile (2 Aug 78):

Dry density of soil, γ	25	1.6 g cm ⁻³		
Specific gravity, G _s	25	2.7		
Gravimetric water content, w	25	18%		
Volumetric water content, $V_{\mathbf{w}}$	25	29%	(Refer to block	
Volume of solids, V	25	60%	diagram, Fig. 4)	
Volume of voids, V	25	40%		
Volume of air, V	25	11%		
Saturation, S	25	73%		

1. Effect of variation in gravimetric water content

Example: 1% variation in w(%)

$$w_1 = 18\%$$
 $w_2 = 19\%$
 $\Delta w = 1\%$

$$V_{w}(\%) = \gamma w(\%)$$

$$\Delta V_{w}(\%) = \gamma \Delta w(\%) = 1.6x1 = 1.6\%$$

In this case, a 1% change in w results in a 1.6% change in V.

In terms of the total amount of water applied, this is equivalent to:

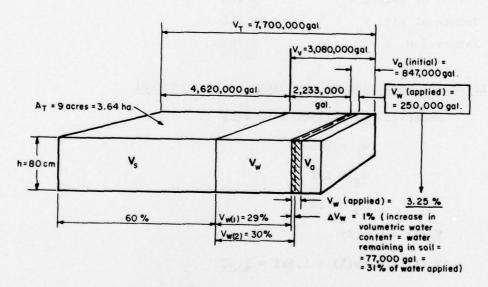


Figure 4. Block diagram of volume relationships.

A 0.5% change in w would result in a 0.8% change in V , or almost 25% of the total water applied. A 0.1% change in w would represent a 0.16% change in $V_{\rm W}$, or almost 5% of the water applied.

2. Effect of variation in dry density

Example: 0.05 g cm^{-3} variation in γ

 $\gamma_1 = 1.55$

 $\gamma_2 = 1.60$

 $\Delta \gamma = 0.05$

$$\Delta V_{\mathbf{w}}(\%) = \Delta \gamma \mathbf{w}(\%) = 0.05 \times 18 = 0.9\%$$

In this case, a 0.05 g cm $^{-3}$ change in γ results in a 0.9% change in $V_{_{\bm W}}.$ In terms of the total amount of water applied, this is equivalent to:

$$V_{w}(gal) = V_{T}(gal) \times \Delta V_{w} =$$
= 7,700,000 gal x 0.009 =
= 69,300 gal, or approximately 28% of the water applied

A 0.01 g cm $^{-3}$ change in the mean profile density would result in a 0.18% change in $\rm V_w$, or 5.6% of the total water applied.

3. Effect of variation in volumetric water content

The mean V values for the soil profile during the 2 August 78 application were in the 27 to 33% range. It has been shown above that a change of 0.5% in the water content (w) or a change of 0.05 g cm $^{-3}$ in the dry density of the soil (γ) results in a change of almost 1% in the volumetric water content (V). Therefore, a 1% variation in V is probably a realistic error to be expected due to measurement and analysis techniques.

Example: 1% variation in V (refer to Fig. 4)

$$v_{w(1)} = 29\%$$

$$V_{w(2)} = 30\%$$

$$\Delta V_{xx} = 1\%$$

In terms of the total amount of water applied, this is equivalent to:

$$V_w(gal) = V_T(gal) \times \Delta V_w =$$

$$= 7,700,000 \times 0.01 = 77,000 \text{ gal, or}$$

$$= 31\% \text{ of the water applied}$$

The effect on saturation due to a 1% change in $V_{\rm W}$, using $V_{\rm V}$ = 40%, would be:

$$S = V_w \div V_v$$

 $S_1 = 29 \div 40 = 72.5\%$
 $S_2 = 30 \div 40 = 75\%$
 $\Delta S = 2.5\%$

A 0.5% change in the volumetric water content would be equivalent to approximately 15% of the total water applied, and would result in a change of a little over 1% in saturation.

4. Effect of variation in specific gravity of soil

Typical specific gravity values of the soil at this site are in the $2.66\ \text{to}\ 2.76\ \text{range}$.

Example: 0.05 variation in G

$$G_{s(1)} = 2.66$$
 $G_{s(2)} = 2.71$
 $\Delta G_{s} = 0.05$

The resulting effect on the volume of solids (or the volume of voids) would be (using $\gamma = 1.6$ g cm⁻³):

$$V_{s} = \gamma \div G_{s}$$
 $V_{s(1)} \sim 60\%; V_{v(1)} \sim 40\%$
 $V_{s(2)} \sim 59\%; V_{v(2)} \sim 41\%$
 $\Delta V_{s} \sim 1\% \Delta V_{v} \sim 1\%$

The effect on saturation due to a 0.05 change in G_s would be (using $V_{st} = 29\%$):

$$S = V_w \div V_v$$

 $S_1 = 29 \div 40 = 72.5\%$
 $S_2 = 29 \div 41 = 70.7\%$
 $\Delta S = 1.8\%$

5. Effect of variation in the amount of water drained through the underdrain system

The estimated maximum error caused by the method of monitoring the flow at Point 5 during the 2 August 1978 application and the integration of the area under the flow rate versus time curve would not ordinarily exceed 5%. The amount of water drained off after 44 hrs was, in this case, 35,500 gal. A 5% error would cause a variation of:

In computing the water mass balance, an error of less than 1% of the total water volume can be considered insignificant.

6. Effect of variation in the evapotranspiration

Since the ET value is not a direct measurement, but is computed or estimated from pan evaporation data based on climatological conditions, it is felt that an error as high as 10% of the total water volume applied is possible.

Summary (for wastewater application of 2.6 cm)

Parameters	Variation (∆)	Resulting vari- ation in V (ΔV _w)	% of total water ap- plied
Water content, w(%)	0.1%	0.16%	5%
	0.5%	0.8 %	25%
	1.0%	1.6%	49%
Dry density, $\gamma(g \text{ cm}^{-3})$	0.01 g cm^{-3}	0.18%	5.6%
	0.05 g cm^{-3}	0.9 %	28%
Volumetric water content, V _w (%)		0.5% 1.0%	15% 31%

Therefore, for a 2.6-cm water application on a 3.64-ha area, a 1% change in the volumetric water content is equivalent to almost 1/3 of the total water applied. For a 5.2-cm application, a 1% change in V would be equivalent to only 1/6 of the water applied.

If it is desired to keep the water mass budget computations within a 20% error (10% due to errors in each, w and γ), the volumetric water content data have to be kept to within a 0.65% error, requiring the water content (w) data to be within a 0.2% error and the dry density data within a 0.02 g cm $^{-3}$ error, not considering errors in computing ET.

If the contributing error from estimating the ET is 10%, the drainage amount error is considered insignificant, and the error in computing the amount of applied water remaining in soil is to be kept at 10% (in order for the cumulative error not to exceed 20%), the water content and dry density errors have to be limited to half of the values shown above.

Example:

Error in w: 0.1% = 0.16% error in
$$V_w(\%)$$

Error in γ : 0.01% g cm⁻³ = 0.18% error in $V_w(\%)$
Total: 0.34% error in $V_w(\%)$
0.34% error in $V_w(\%)$ = 7,700,000 gal x 0.0034 = 26,180 gal
10% error in computing ET = 250,000 gal x 0.1 = 25,000 gal
Total: 51,180 gal \sim

 $\stackrel{\sim}{\sim}$ 20% of total water applied

Therefore, great care has to be exercised in the field measurements, data plotting and analysis, if the desired accuracy of the mean water content profile is to be kept to \pm 0.1%, and that of the dry density profile to \pm 0.01 g cm $^{-3}$, in order to stay within a 10% error of the total water volume applied. This degree of accuracy may, however, be difficult to achieve in field measurements.

The relationship between dry density of soil, gravimetric water content, volumetric water content and the resulting total volume of water for the 3.64-ha (9-acre) test field is shown in the nomogram (Fig. 5).

The example shown in the nomogram is as follows: for a dry density of $1.6~\rm g~cm^{-3}$ and a gravimetric water content of 18%, the resulting volumetric water content is 28.8%, representing a total volume of water of $8.37~\rm Ml~(2.21~\rm Mgal)$ in the test field. An increase in the gravimetric water content to 19% corresponds to a volumetric water content of 30.4% (an increase of 1.6%), representing a water volume of approximately $8.82~\rm Ml~(2.33~\rm Mgal)$, or an increase of approximately $0.45~\rm Ml~(0.12~\rm Mgal)$. The increase in the volumetric water content (%) in terms of the actual volume of water (l or gal) or the equivalent percentage of total water applied, is shown in Figure 6.

It should be pointed out that in this case an error or variation of only 0.33% in the volumetric water content is just as serious as a 10% error in ET.

Field Data

Soil density and specific gravity. The dry density profile data (Table 2) obtained from the three test sites are plotted in Figure 7. The scatter of data is quite significant, indicating that the soil density profile varies considerably throughout the test area. The scatter may also be caused by different measurement techniques or by differences in the water content during density measurements due to shrinkage or swelling of the soil because of its high clay content. The dashed line, constructed by computing the mean density at the various depths where data were available, represents the mean dry density profile of the test area. The mean density for the 0 to 80 cm depth of soil, integrated at 5-cm increments, is 1.59 g cm⁻³. The mean density values for the individual test plots are as follows:

Grass plot: 1.58 g cm₋₃
Soybean plot: 1.58 g cm₋₃
Alfalfa plot: 1.60 g cm

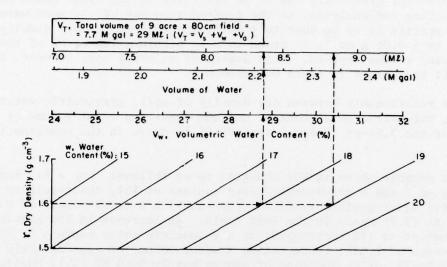


Figure 5. Relationship between dry density, gravimetric water content, volumetric water content, and volume of water in soil.

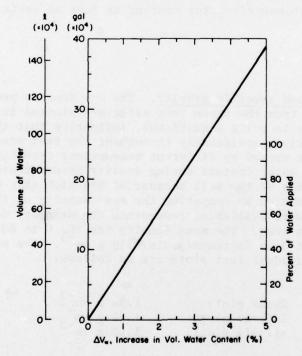


Figure 6. Volume of water vs increase in water content.

It is interesting to note that, although the variation in data (width of the data envelope in Fig. 7) at any particular depth is, for the most part, greater than $0.2~{\rm g~cm}^{-3}$, the variation between the 0 to 80-cm mean density values for the three test plots is only $0.02~{\rm g~cm}^{-3}$ (two of the mean values being identical).

The specific gravity data (Table 2) are plotted in Figure 8. The mean G value for the 0 to 80-cm depth is 2.71 (integrated at 5-cm increments). It was assumed that the 20-cm value is representative of the 0 to 20-cm depth.

Soil water content. The mean soil water content (w) profiles for the three test plots prior to the 2 August wastewater application are shown in Figure 9. The profiles of the grass and alfalfa test plots are reasonably similar. The soybean plot data were obtained from the pit at one end of the test plot (refer to Fig. 2) slightly outside the spray area. The soybean plot water content profile below 35 cm is significantly different from the profiles of the grass and alfalfa plots. (The initial saturation values for 0 to 80-cm depth were: grass plot: 65%; alfalfa plot: 68%; soybean plot at pit site: 80%.)

The corresponding volumetric water content (V) profiles are plotted in Figure 10. The V values were computed by using the density data for each individual test plot (Table 2), not the mean density profile shown in Figure 7.

For the water budget analysis, only the water content data from the grass and alfalfa plots were used as representative of the 3.64-ha (9-acre) test area, since 1) no water content data were obtained in the soybean plot during or after the wastewater application because of time constraints, and 2) the data from the pit may not have been representative of the initial (before application) water content profile of the soybean plot.

The actual data points of the initial water content in the grass and alfalfa plots (Table 4) on 2 Aug are plotted in Figure 11a; Figures 11b through 11d show the water content during and after the wastewater application. The lines in Fig. 11 represent the mean profiles from the combined grass and alfalfa data.

It is apparent that the 8.5 hour water content data (2 hrs after end of application) are lower than those before, during, or 2 days after application. The available data are not adequate to explain this.

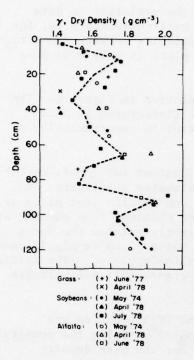


Figure 7. Dry density profile of test area.

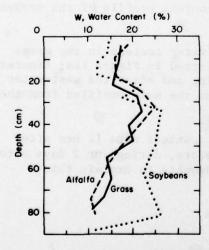


Figure 9. Gravimetric water content before application.

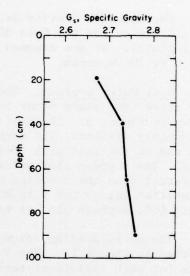


Figure 8. Specific gravity profile of test area.

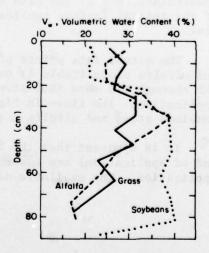


Figure 10. Volumetric water content before application.

The mean water content values (combined data from the grass and alfalfa plots) are as follows:

Time			Mean w(%)
Before	application	(0 hrs)	17.18
During		(4 hrs)	18.30
After		(8.5 hrs)	16.35
"		(44 hrs)	17.94

Consequently, the water budget calculations were limited to the 4-and 44-hour data.

The water content (w) profiles for 0 and 4 hrs are compared in Figure 12a, and for 0 and 44 hrs in Figure 12b. The corresponding volumetric water content (V) profile comparison is shown in Figure 13. It appears that the depth increments for the 4 and 44-hr water content data (approximately 15 cm between samples) may have been too large, possibly causing some of the high water content locations to be missed (at 25, 35, and 45 cm, for example). This problem is even more apparent in Figure 14, where all three components of the soil profile (volume of solids, volume of water, and volume of air) are shown.

If V + V profile lines were plotted from the available data for 4 and 44 hours and compared with the V + V profile before application, it would appear that at certain depths (at 10 to 15 cm and at 25 to 30 cm, in particular) the volume of water had decreased after application. This impression is due to the lack of data for the 4 and 44-hour water content conditions at certain specific depths where the before-application data (obtained at approximately 5-cm increments) indicate noticeable peaks in the V + V profile (refer to Fig. 14).

Therefore, it is evident that water content data obtained at 15-cm increments with depth may not be sufficient to provide accuracy of the V + V profile to a 5% level. Data obtained at 5-cm increments would increase the profile accuracy. Also, it is very important that the data at the various time intervals be obtained at the same depth in order to permit a reliable comparison of the "before" and "after" water content conditions. The question still remains whether or not the sampling done at one location in each plot is representative of the entire plot.

Drainage rate. A record of the drainage flow rate at the underdrain discharge point (Point 5; refer to Fig. 2) and the wastewater application and precipitation data are shown in Figure 15. A total of almost 2 cm of rainfall occurred shortly prior to and during the 10 August application, resulting in an exceptionally high peak flow.

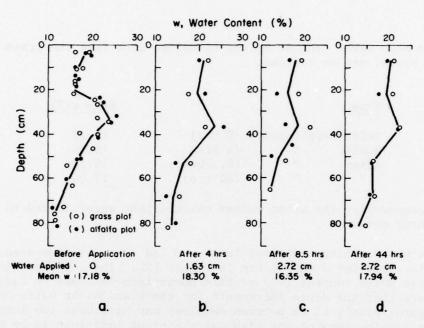


Figure 11. Gravimetric water content profiles before, during, and after application.

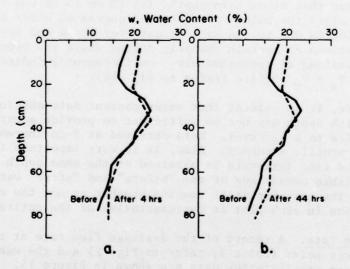


Figure 12. Mean gravimetric water content profiles of test area.

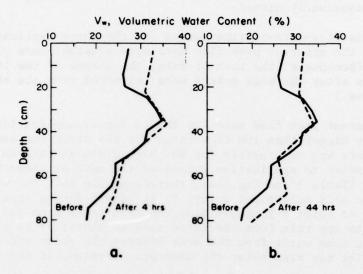


Figure 13. Mean volumetric water content profiles of test area.

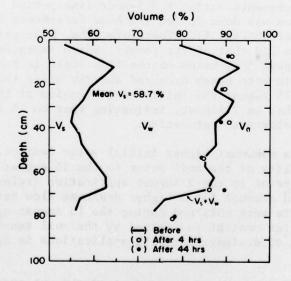


Figure 14. Volumetric composition of test area.

During the 22 August and 7 September applications, the peak flow points were apparently missed.

The drain flow-rate-vs-time curves for the three applications during which the apparent peak flow data were available are plotted in Figure 16. (Because of the lack of data, the shapes of the 10 and 15 August curves after the peak points were estimated from the shape of the 2 August curve.)

The apparent peak flow rate for the 10 August application is significantly higher than the flow rates for the other two applications. Several factors are responsible for the high drainage rate in this case: 1) rainfall prior to application saturated the soil as shown by the soil tension data (Table 3 and Fig. 16); therefore, the soil had very little capability to hold additional water; 2) it rained during application and the drainage at Point 5 includes also the rain water from the tree plot, in addition to the rain from the three sprayed plots; 3) it may be possible that some water from the area between the plots and the drainage discharge point may also enter the underdrain system, if the drain pipes are cracked.

It is not completely clear why the peak flow rate for the 15 August application is twice as high as that for the 2 August application. The amount of wastewater applied in both cases was approximately the same, and rainfall was not a factor. The rate of application and the initial water content were the principal differences the 2 August application was done in 5 increments during a 6.5-hour time period, while the 15 August application was done in ten 1/2 hour increments during a 9.7-hour time period (refer to Fig. 3). Therefore, the mean rate of application of the latter was 2/3 that of the former, which explains the relative location of the peak flow rates on the time scale in Figure 16. The peak flow rates in both cases occurred shortly after the end of application. Also, on 15 August the initial soil tension at the 25 cm depth was lower than that on 2 August, indicating that on 15 August the soil below 25 cm was closer to saturation.

Because of a somewhat higher initial water content, the water retention capability of the soil prior to the 15 August application was lower than that prior to the 2 August application (refer to soil tension data) which would account for a higher drainage flow rate. No direct water content data were obtained during the 15 August application; the difference in water content is implied by the soil tension data. After 2 days, the rate of drainage for both applications is approximately the same.

The cumulative amount of water drained through the underdrain system, plotted vs. time for the 2 and 15 August applications, is compared with the amount of water applied in Figure 17. After 1 day,

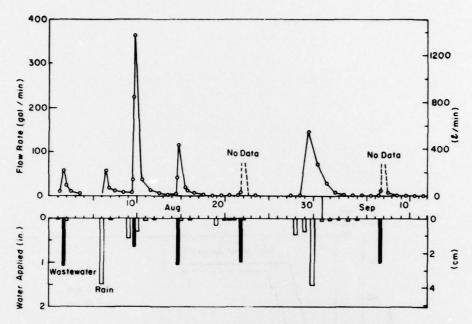


Figure 15. Drain flow rate compared with wastewater applied and rain.

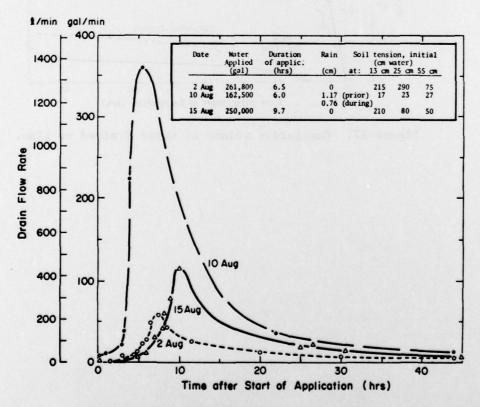


Figure 16. Drain flow rate vs time.

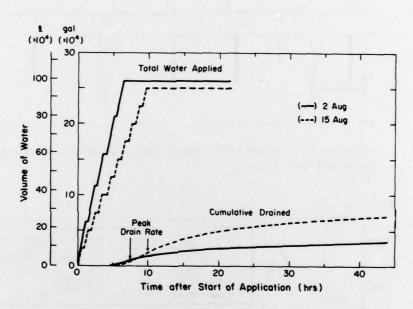


Figure 17. Cumulative volume of water drained vs time.

the cumulative drainage for the 15 August application is approximately twice that of the 2 Aug application.

The vertical distance between "total water applied" and "cumulative drained" lines in Fig. 17 indicates the amount of water remaining in the soil and lost due to evaporation and transpiration.

The cumulative drainage figures for the 2, 10, and 15 August applications at various times after the start of application are compared in Table 7.

Table 7. Cumulative drainage

Date	Time	(hrs):	4	8	12	24		% of applied (44 hrs)
				Amount of	water dra	ined (gal)		
2 Aug		1	,400	10,740	17,940	28,350	35,350	13.6%
10 Aug		18	,480	92,280	133,080	174,180	197,340	*
15 Aug			600	6,960	29,160	55,680	70,320	28.1%

^{*}Not determined due to uncertain amount of rain.

Evapotranspiration. The percentage of water lost to ET during the 2 August application was estimated from methods discussed in literature (Chang, 1968; Bilello and Bates, 1978). The usual method for estimating ET (80% of pan evaporation during the day of application) resulted in a value of approximately 0.5 cm (80% x 0.26 in; refer to Table 6), or approximately 18% of the water applied. The ET during the next day was estimated from transpiration vs. volumetric water content relationship (Chang, 1968), resulting in a value of less than 0.25 cm, or approximately 10% of the water applied. Therefore, the total estimated ET after 44 hours was 28% of the water applied. During application (after 4 hours) the ET was estimated to be 1%.

Water Budget Analysis

The relationship of the principal components in the water budget can be expressed by:

Water applied + precipitation = water drained +

- + water remaining in soil +
- + water lost due to evapotranspiration

Other components, such as water in the drainage and spray systems and in the soil below the drains were considered insignificant for calculating the total budget.

The amount of water drained through the underdrain system, obtained by integrating the area under the 2 August curve in Figure 16 (the curve was replotted on an expanded scale for area integration), was:

> After 4 hrs: 1,400 gal After 44 hrs: 35,500 gal

The estimated amount of water lost to evapotranspiration was:

After 4 hrs: $1\% \times 156,900$ (gal applied) = 1,600 gal After 44 hrs: $28\% \times 261,800$ (gal applied) = 73,300 gal

Therefore, the total amount of water lost due to drainage and evapotranspiration (D + ET) was:

After 4 hrs: 3,000 gal After 44 hrs: 108,800 gal

The amount of water remaining in soil was computed by a number of methods, depending on the procedure used for calculating the water content from the data available. Although the variations in the water content values between the results of the various computation methods appear to be very small numerically, their effect on the total water budget can be quite significant, as discussed previously under "Sensitivity analysis."

Six methods of computation were used for determining the amount of water remaining in the soil after 4 and 44 hours. The first three methods used the gravimetric water content profiles and the mean dry density values, while the other three methods used the volumetric water content profiles. The results are listed in Table 8.

Method No. 1: The gravimetric water content profiles were plotted separately for the grass, alfalfa, and soybean plots (Fig. 9); the mean value of each profile was calculated by using values from each profile at 5-cm depth increments and then multiplied by the corresponding mean dry density profile value (Fig. 7) to obtain the mean volumetric water content value. Since water content data during and after application were obtained only on the grass and alfalfa plots, the mean of these two profiles at 0, 4, and 44 hrs was used as the mean water content in the 3.64-ha (9-acre) test field.

Method No. 2: One gravimetric water content profile was plotted using the combined grass and alfalfa data for each observation time (Fig. 11) and the mean value for each profile computed as in Method No. 1, using 5-cm-depth increments and the mean dry density profile value.

Theoretically, both methods would give the same results, if the same number of data points were obtained at the same depth for both test plots. Since this was not the case, there is some variation between the results from the two methods.

- Method No. 3: The areas between the mean 0-hr and 4-hr and between the 0-hr and 44-hr gravimetric water content profiles (Fig. 12) were integrated by measurement at 2-cm-depth increments, and these values were multiplied by the mean dry density to obtain the corresponding difference in the volumetric water content.
- Method No. 4: The gravimetric water content values were taken at 5-cm increments from the mean water content profiles (Fig. 11) and multiplied by the corresponding dry density values, resulting in a volumetric water content profile for each of the 0, 4, and 44-hr observation times (Fig. 13). The mean value of each profile was then calculated, using the values at 5-cm-depth increments.
- Method No. 5: Comparisons between the mean initial volumetric water content profile and those of 4 and 44 hours were made at those depths where actual data were obtained after 4 and 44 hours; i.e., the mean value of each profile was calculated using approximately 15-cm-depth increments.
- Method No. 6: Same procedure as in Method No. 3, except the areas integrated (measured) were from the volumetric water content graphs (Fig. 13), instead of the gravimetric water content graphs.

The values representing the amount of water remaining in soil after 4 and 44 hours, computed by using the six analysis methods, are summarized in Table 8.

The area integration methods (Nos. 3 and 6) resulted in values that are too high at 4 hours; i.e., the amount of water remaining in soil was greater than the amount applied during the first 4 hours of spraying. This was also the case with Method No. 4. In general, calculations using the volumetric water content data (Methods 4 through 6) resulted

Table 8. Results of calculations for water remaining in soil.

Analysis	Time	w(%)	Y (g cm ⁻³)		Water remaining in soil				
				V _w (%)	Δ V _W (%)	Δ V _w (gal)	D+ET (gal)	Total (gal)	% of applied
(Grass)	Before	17.03	1.58	26.91	-				
	4 hrs 44 hrs	18.47 19.00	1.58 1.58	29.18 30.02	2.27 3.11				
(Alfalfa)		17.53	1.60	28.05	-	100110			
	4 hrs	18.34 17.66	1.60	29.34 28.26	1.29 0.21				
No. 1									
(Mean)	Before 4 hrs	17.28 18.41	1.59	27.48	1.79	137,800	3,000	140,800	89.7
	44 hrs	18.33	1.59	29.14	1.66	127,800	108,800	236,600	
<u>No. 2</u>	Before	17.18	1.59	27.32	1.78	-	- 000	140 000	
	4 hrs 44 hrs	18.30 17.94	1.59 1.59	29.10 28.52	1.78	137,000 92,400	3,000 108,800	140,000 201,200	
No. 3	Before		1.59	-	:				
	4 hrs 44 hrs	Δw=1.35 Δw=1.25	1.59 1.59	•	2.15 1.99	165,500 153,200	3,000 108,800	168,500 262,000	
No. 4	Before			27.25	-	-			
	4 hrs			29.30 29.23	2.05 1.98	157,800 152,500	3,000 108,800	160,800 261,300	
No. 5	Before			26.84					
	4 hrs 44 hrs		:	28.62 28.56	1.78 1.72	137,000 132,400	3,000 108,800	140,000 241;200	
<u>No. 6</u>	Before								
	4 hrs 44 hrs		·	30.11	2.45 2.30	188,600 177,100	3,000 108,800	191,600 285,900	
Mean	Before 4 hrs 44 hrs		seas La Laborate	27.2* 29.1* 28.9*	2.00 1.81	154,000 139,200	3,000 108,800	157,000 248,000	

^{*} Mean of 4 analyses only

in higher values than those using the gravimetric water content and mean dry density data (Methods 1 through 3).

Assuming that the measurement as well as the analysis errors are as likely to be positive as negative, the logical approach for computing the most realistic value of the amount of water remaining in soil appeared to be the use of the mean value from the six separate analysis methods. By doing that, it was possible to account for all of the water applied at 4 hours and approximately 95% of the water two days after application (Table 9).

These results illustrate the great sensitivity of the results to the method of analysis or computation procedure used. At 4 hours after the beginning of application, for example, three analysis methods result in values that are too low, and the other three methods give values that are too high, but the mean value of all six corresponds to the exact theoretical value of the amount of water remaining in the soil (i.e., water in soil = water applied - water drained - water lost to ET).

At 44 hours after the beginning of application, the amount of water unaccounted for is approximately 5%. The values for the percentage of water accounted range from approximately 77% to approximately 109% from the six analyses, the mean being 94.8% (refer to Table 8).

The volumetric water budget values for the 2 Aug wastewater application are summarized in Figure 18. The bar graph on the left shows the volumetric composition of the test field during (at 4 hrs) the wastewater application. The one on the right shows the volumetric composition after 44 hours. The applied water volume is also illustrated on an expanded scale to show the distribution of the applied water.

Infiltration Rate

Results of the infiltration test conducted near the pit at one end of the soybean plot are shown in Figure 19. A total of approx. 5.1 cm of water was applied to the test area. It is estimated from evaporation data that approximately 0.5 cm of water was lost to evapotranspiration. After 24 hours, there was no free water (ponding) on the soil surface. The saturation of the soil prior to application was approximately 80%. The initial water content profiles at this site are shown in Figures 9 and 10.

During the first hour after application, the rate of infiltration (determined from the slope of the curve in Fig. 19) was approximately 1 cm per hour. After 6 hours, the rate was approx. 0.2 cm per hour, decreasing eventually to approx. 0.1 cm per hour. According to the USDA-SCS permeability classification for saturated soils, these rates would be comparable to a range from moderately slow (1 cm hr $^{-1}$) to very slow (0.1 cm hr $^{-1}$) infiltration.

Table 9. Summary of water budget.

		After 4 hours			After 44 hours		
		ΔV _W (%)	V _w (gal)	% of applied	∠V _w (%)	V _w (gal)	% of applied
Water applied (during 6.5 hours)			156,900			261,800	
Water remaining in soil							
Analysis No:	1 2 3 4 5	1.79 1.78 2.15 2.05 1.78			1.66 1.20 1.99 1.98 1.72 2.30		
Mean:	0	$\frac{2.45}{2.00}$	154,000	98%	1.81	139,200	53.2%
Water drained:			1,400	1%	day	35,500	13.6%
Evaportranspiration:			1,600	1%		73,300	28%
Total:			157,000	100%		248,000	94.8%
Unaccounted:			-			13,800	5.2%

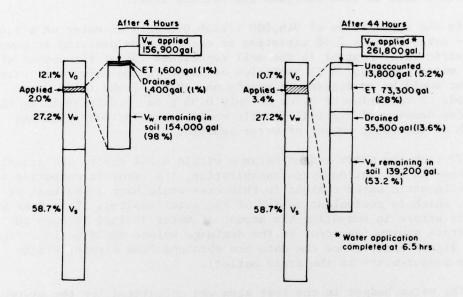


Figure 18. Volume relationships and water budget (2 Aug 1978).

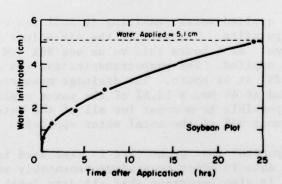


Figure 19. Infiltration rate.

SUMMARY AND CONCLUSIONS

Computation of the water budget values during and after water application is very sensitive to the accuracy and variations in the field measurements of water content and physical soil properties and to the method used in data analysis and calculations.

In the application of 946,000 ℓ (250,000 gal) of water on a 3.64-ha (9-acre) area, a 0.5% variation or error in the measured or computed volumetric water content in the soil represents 15% of the amount of total water applied. A 1% variation or error in the volumetric water content would be equivalent to nearly one third of the total water applied. A variation or error of only 0.05 g cm⁻³ in determining the mean dry density of the soil profile would represent more than one fourth of the total amount of water applied.

To keep the water budget balance within a 20% error, and assuming a 10% error in estimating evapotranspiration, the error in computing the soil volumetric water content in this case would have to be kept at 0.32%, which is equivalent to 10% of the water applied. This does not include errors in computing the amount of water drained through the underdrain system (an error in the drainage volume would be less likely to be significant, since the data are obtained from direct, single source measurements at the drain outlet).

The water budget in the test area was calculated for the wastewater applied on 2 August. A total of 991,000 ℓ (261,800 gal) equivalent to 2.72 cm (1.07 in) on a 3.64-ha (9-acre) area, were applied during a period of 6.5 hours. The calculations were done for the water budget at 4 hours (during application) and at 44 hours (almost 2 days after application).

The volume of applied water remaining in soil was calculated using six methods of computation. The mean of the six values was used as the representative value. At 4 hours this value was 98% and at 44 hours 53.2% of the water applied. The evapotranspiration was estimated to be 1% at 4 hours and 28% at 44 hours. The drainage measured at 4 hours was approximately 1% and at 44 hours 13.6% of the water applied. Therefore, at 4 hours it was possible to account for all of the water applied and at 44 hours for almost 95% of the total water applied.

It is very important that great care is exercised in obtaining the soil water content data in order to achieve reasonably accurate water budget values. It is also important that all data (soil density and water content) in all observations and tests be obtained at the same depth to permit a more realistic and accurate comparison of the soil characteristics before, during and after water application.

The accuracy in computing the water mass balance would increase proportionally with an increase in the amount of water applied.

An in-situ infiltration test conducted on the soybean plot indicated that on the day of the test (31 July 1978) the infiltration rate at this site (initial saturation approximately 80%), ranged from approximately 1 cm per hour during the first hour after application to approximately 0.1 cm per hour 1 day after application.

LITERATURE CITED

- Bilello, M.A. and R.E. Bates (1978) Climatic survey at CRREL in association with the land treatment project. CRREL SR 78-21.
- Chang, J. (1968) <u>Climate and Agriculture</u>, Chicago: Aldine Publishing Co., pp. 129-144.
- Lambert, D.J. and H.L. McKim (1977) Deer Creek Lake On-land wastewater treatment system. Proceedings of the 9th Annual Meeting on Waste Management, 26-29 April, Cornell University.

APPENDIX: SOIL TENSION VS TIME AND DEPTH

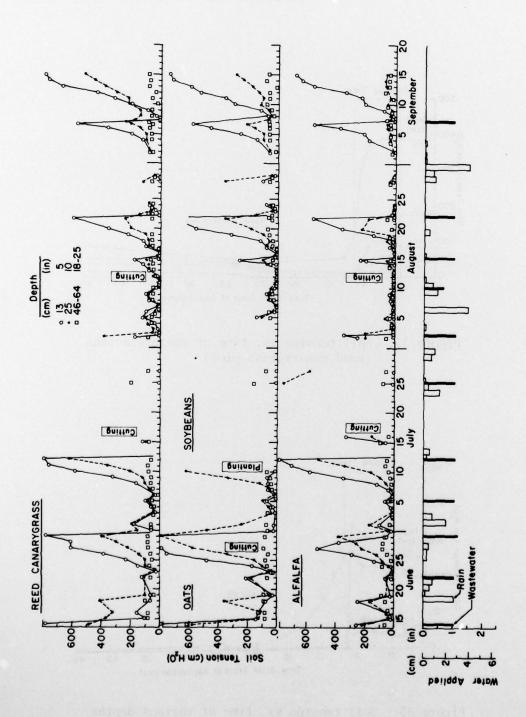


Figure A1. Soil tension vs. time.

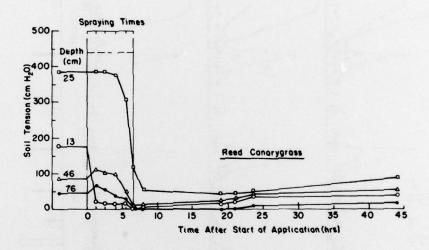


Figure A2. Soil tension vs. time at various depths (reed canarygrass plot)

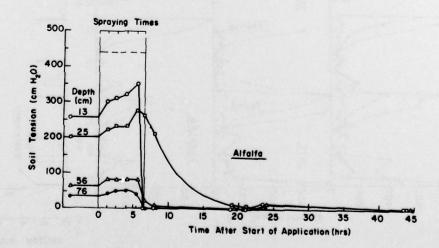


Figure A3. Soil tension vs. time at various depths (alfalfa plot)

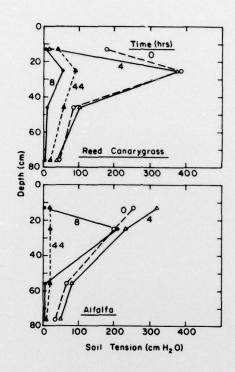


Figure A4. Soil tension vs. depth.